

*The Towers at Greenville Place
Tower 'B'*

Wilmington, DE

Shawn M. Brandt

Structural



Introduction

Existing Structure

- Foundations
- Floor system
- Gravity system
- Lateral system

Proposal and Problem Statement

- Depth Topic
- Breadth Topics
 - Construction Management
 - Sustainability



Outline

Structural Redesign

- Gravity System
- Floor System
- Lateral System
- Weight Comparison
- Seismic Comparison

Construction Management Breadth

- Cost Comparison
- Schedule Comparison

Conclusions, Acknowledgements, Questions & Comments

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Location:

Wilmington Delaware

Owned and managed by:

Pettinaro Real Estate Development Company

Delivery Method:

Design-Bid-Build

Construction Time:

November 2006 – May 2008

Cost:

\$11.5 Million



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Occupancy:

**Upscale Residential Apartment Building
(89 Units)**

Size:

180,000 Square Feet

Stories:

7 Stories above grade

1 Story partially below grade

Major Building Codes:

**IBC and amendments adopted by New Castle
County (DE)**



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Foundations:

3000 psf allowable soil bearing capacity

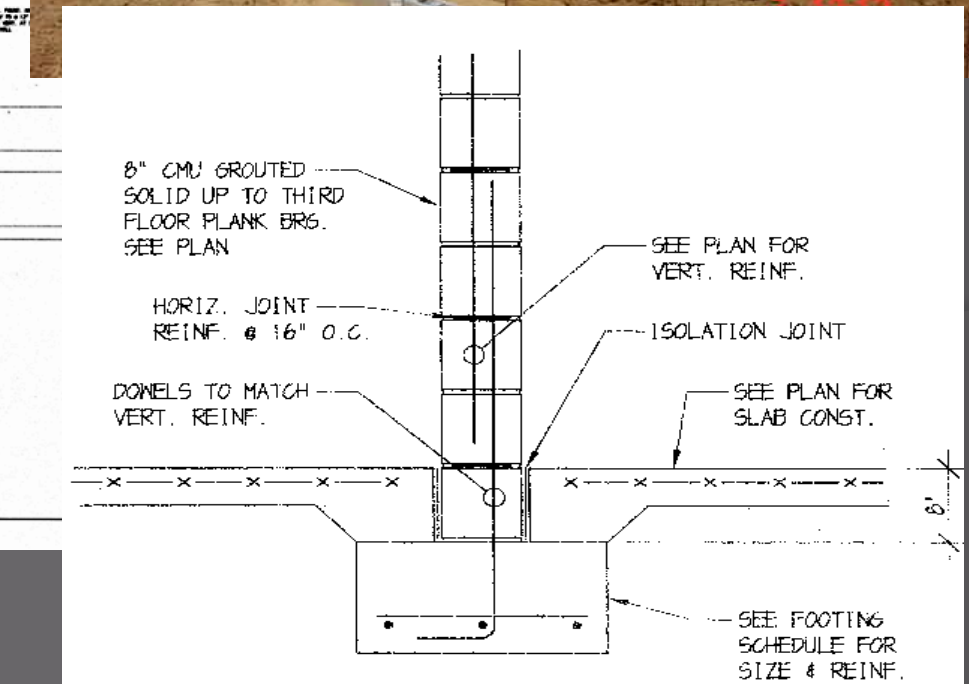
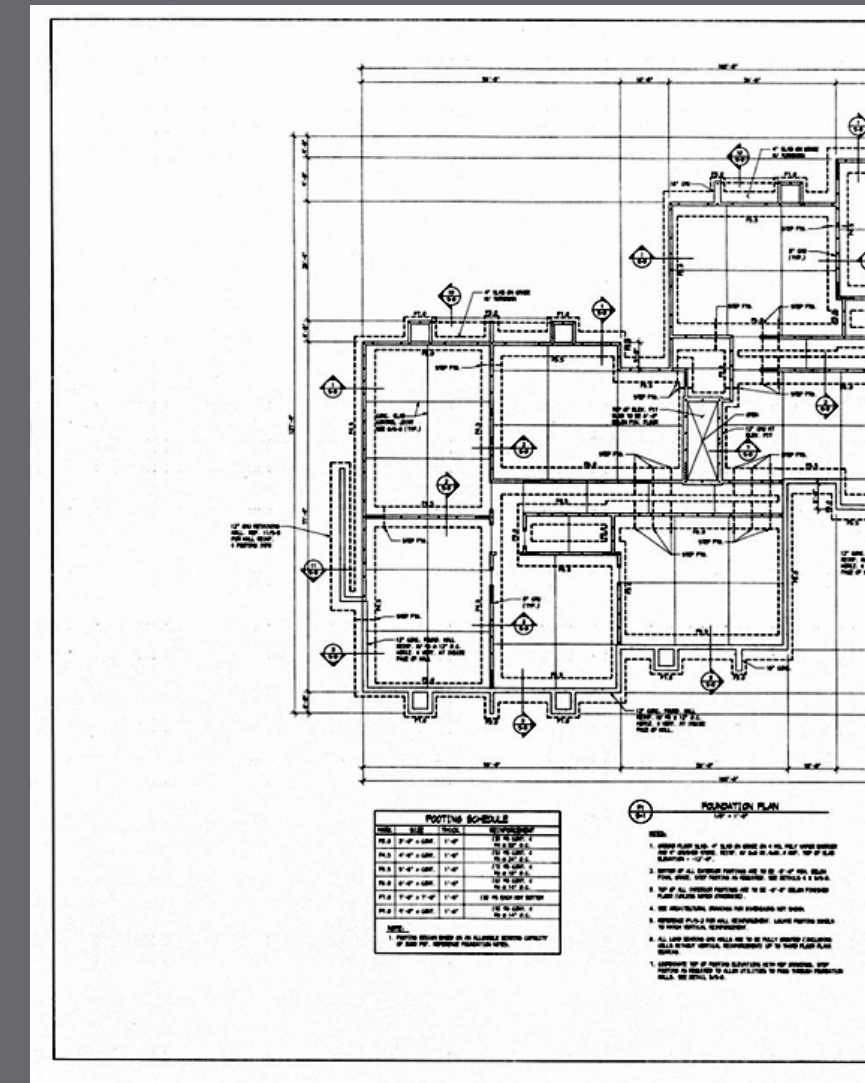
3000 psi concrete spread footings

4 inch thick slab on grade

3500 psi concrete

On 4 inches of crushed stone

6x6 W1.4xW1.4 WWF



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Floor System:

Precast Hollow Core Plank

Rest on 3 inch bearing plate on CMU bond beam

Tied in with #4 reinforcing bars spaced at 48 inches

Technical drawings showing cross-sections of the floor system. The drawings include labels for: HORIZ. JOINT REINF. @ 16" O.C., #5 CONT., GROUT SOLID, 4" CMU. CUT INSIDE FACE SHELL AS REQ'D @ HOOK OR FORM EXT. FACE OF WALL, 8" CMU. SEE PLAN FOR VERT. REINF., #4, 8" PLANK, 3" BEARING STRIP, BOND BEAM W/ (1) #5 CONT., 1" SIDELAP, BREAK OUT PLANK & GROUT SOLID, #4 @ 48" O.C. x 8" PLANK, 8" PRECAST PLANK, SEE PLAN, BOND BEAM W/ (1) #5 CONT., 1" SIDELAP, SECTION @ PLANK BEARING, 3/4" = 1'-0", PLANK PARALLEL TO WALL, 3/4" = 1'-0", SECTION, 3/4" = 1'-0".

Construction photos showing the installation of the precast hollow core planks on the CMU bond beam. One photo shows a worker on a high-rise building with a crane lifting a plank. Another photo shows a worker on a lower level of the building with a crane lifting a plank. A third photo shows a worker on a lower level of the building with a crane lifting a plank. A fourth photo shows a worker on a lower level of the building with a crane lifting a plank. A fifth photo shows a worker on a lower level of the building with a crane lifting a plank. A sixth photo shows a worker on a lower level of the building with a crane lifting a plank. A seventh photo shows a worker on a lower level of the building with a crane lifting a plank. An eighth photo shows a worker on a lower level of the building with a crane lifting a plank. A ninth photo shows a worker on a lower level of the building with a crane lifting a plank. A tenth photo shows a worker on a lower level of the building with a crane lifting a plank. A timestamp '10 13:07' is visible in the bottom right corner of the photos.

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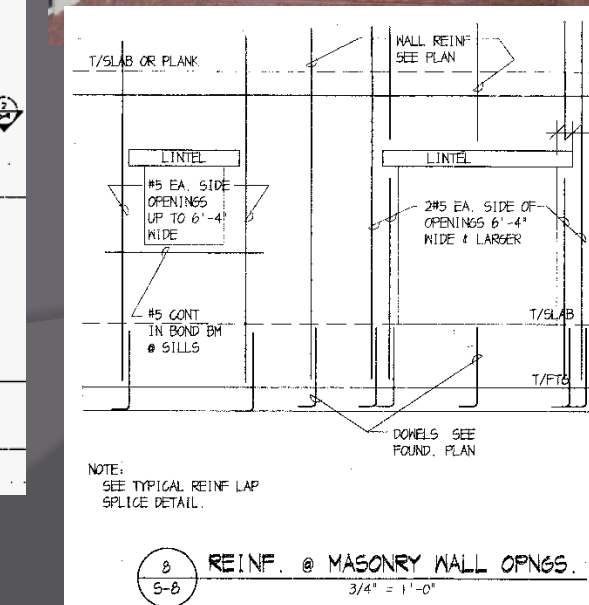
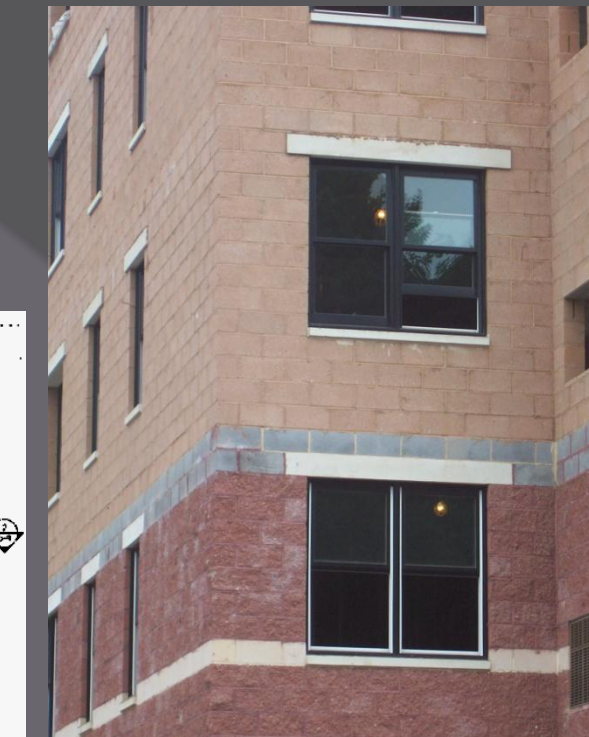
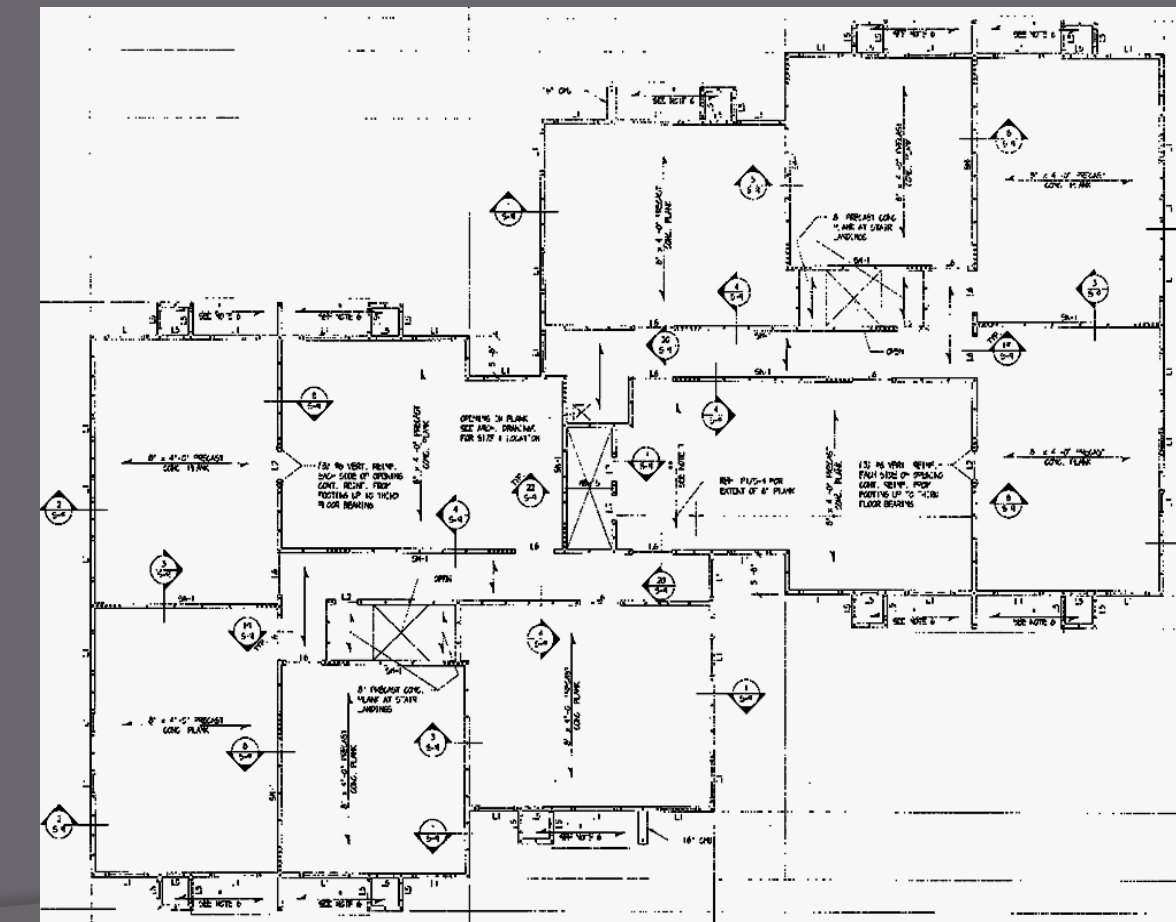
Gravity System:

8 inch CMU

Decorative CMU on exterior walls

#4 reinforcing bars spaced at 32 or 48 inches
(depending on level) in grouted cells

Window and door openings supported by
precast lintels



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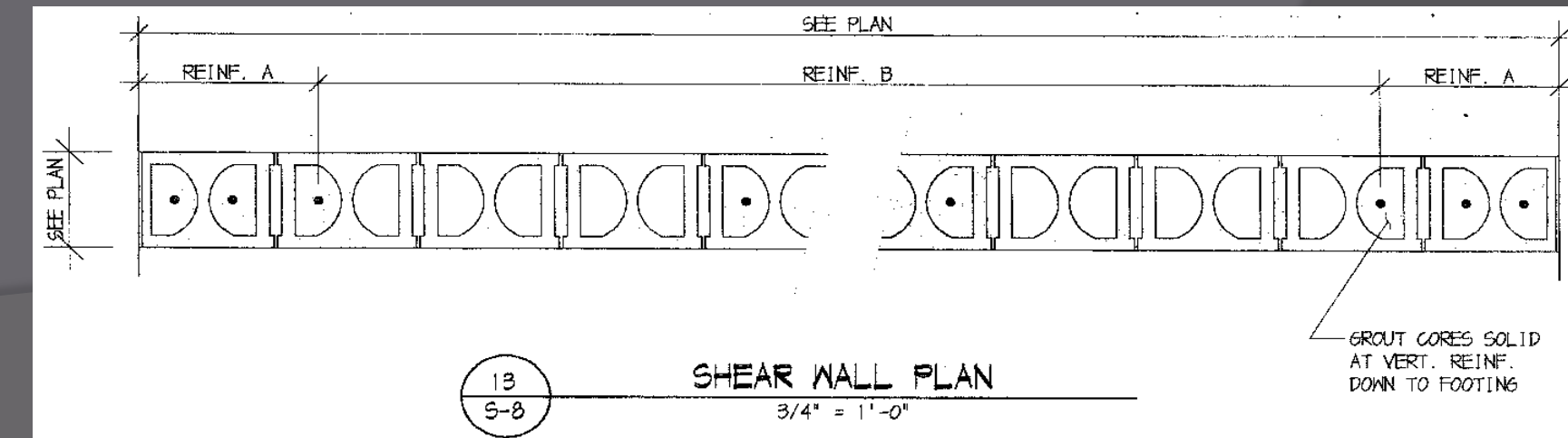
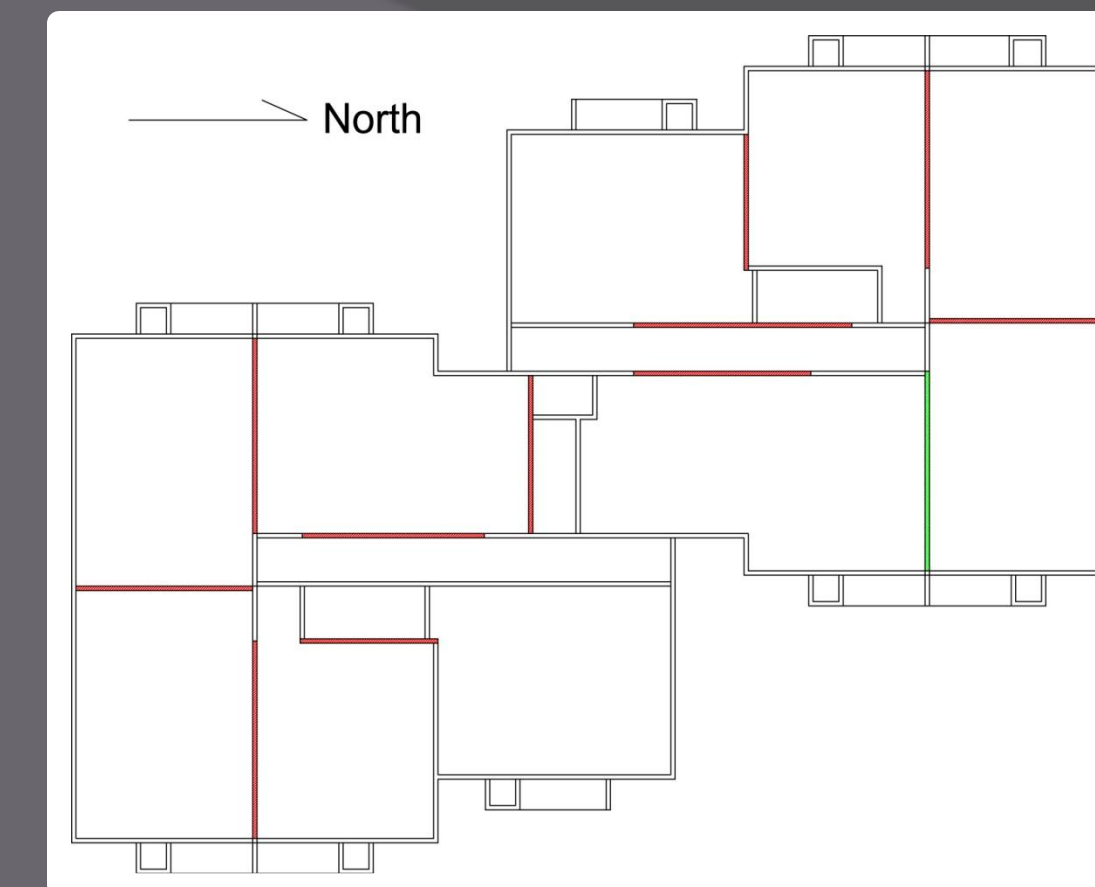


Lateral System:

8 inch CMU walls

Reinforced cells grouted solid all the way down to foundation

Reinforced according to level and load



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Problem Statement:

Large building self-weight

Lateral system design controlled by seismic force

Level	Dead Loads
Roof	821.30
7	1788.24
6	1738.89
5	1738.89
4	1738.89
3	1738.89
2	1738.89
1	1788.24
Ground	1685.56
Total Dead Load	14777.80



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Proposed Solution:

Reduce building self-weight

Redesign entire structural system using the
Infinity Structural System

Reduce seismic load



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Green roof

Rainwater collection

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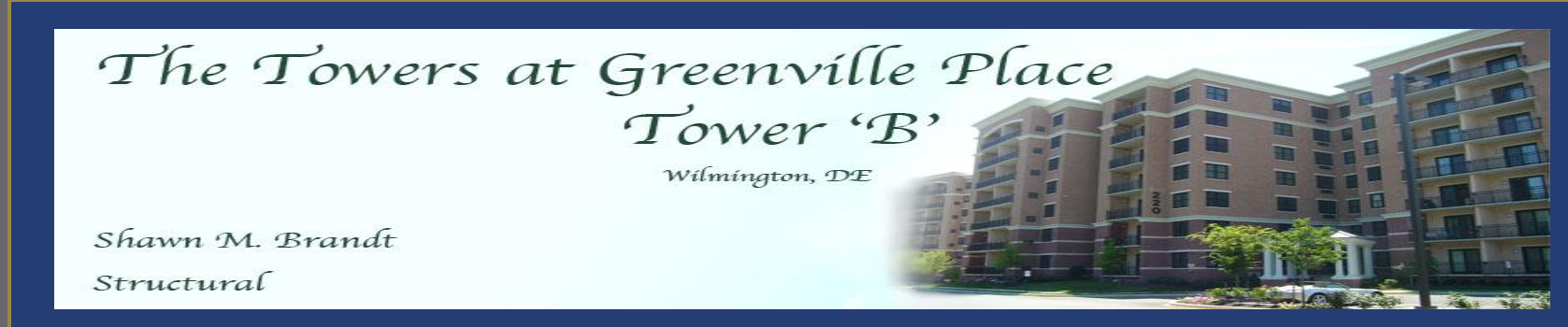
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Depth Topic: Structural Redesign

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Gravity System:

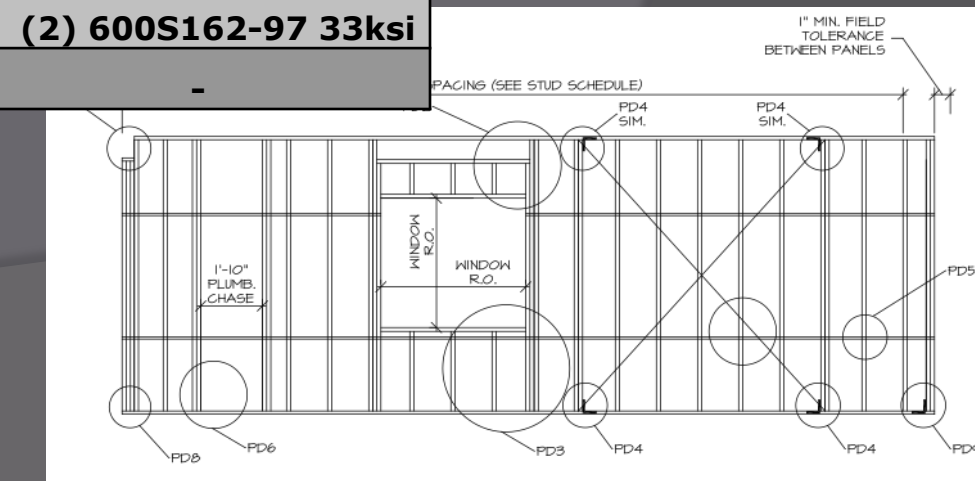
Ground floor structural system would remain similar to existing

“Pre-panelized” metal stud wall system

Additional walls added due to alternative floor system considerations

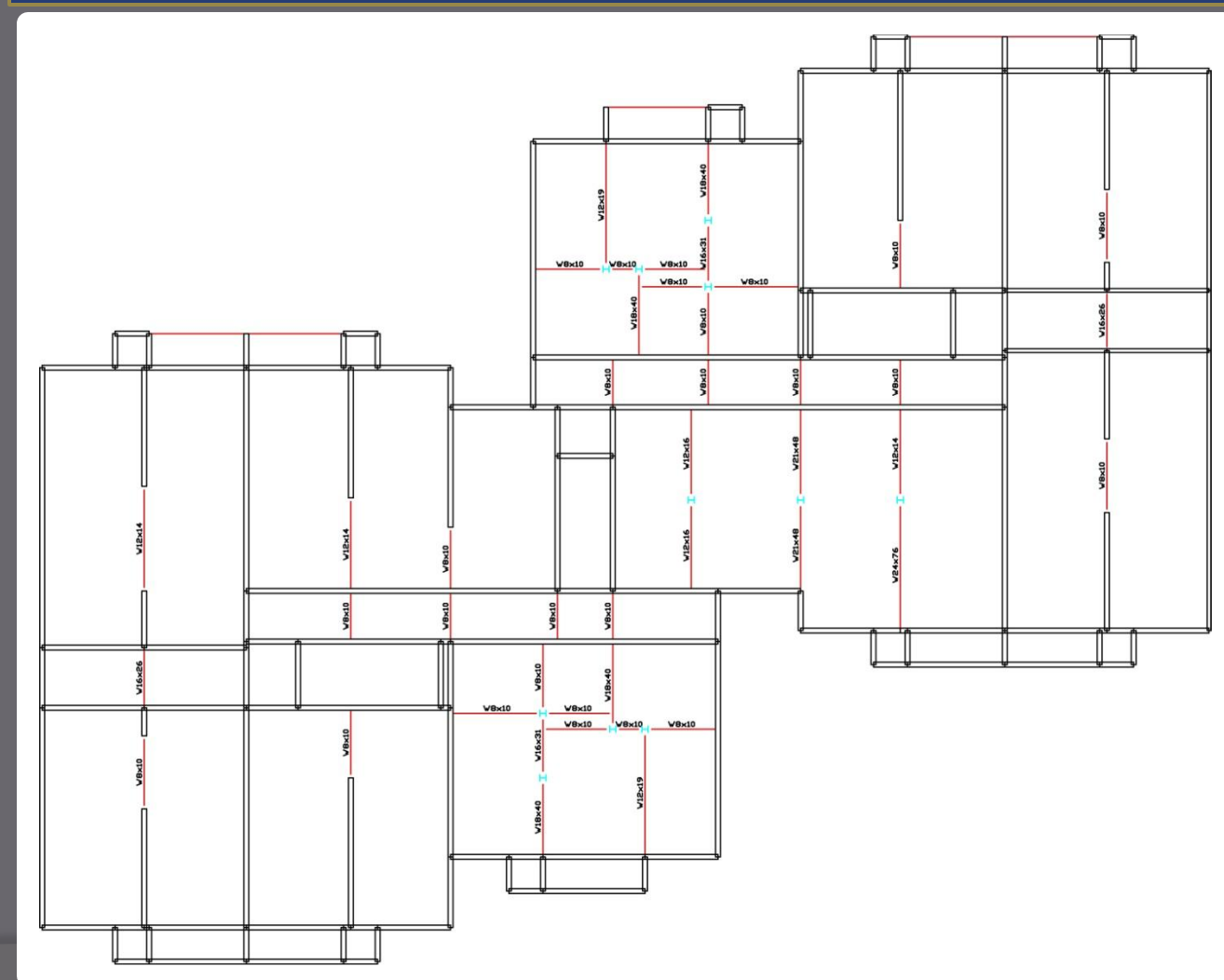
Studs sized according to manufacturer specs

Level	Floor Height	Total Load per Stud(s) (kips)	Stud Spacing (in)	Stud Type
Roof	-	-	-	-
7	10	1.36	16	600S162-54 33ksi
6	9.33	2.84	16	600S162-54 33ksi
5	9.33	4.32	16	600S162-68 33ksi
4	9.33	5.80	16	600S162-97 33ksi
3	9.33	7.28	16	600S162-97 33ksi
2	9.33	8.76	16	(2) 600S162-97 33ksi
1	10	10.24	16	(2) 600S162-97 33ksi
Ground	12	11.72	-	-

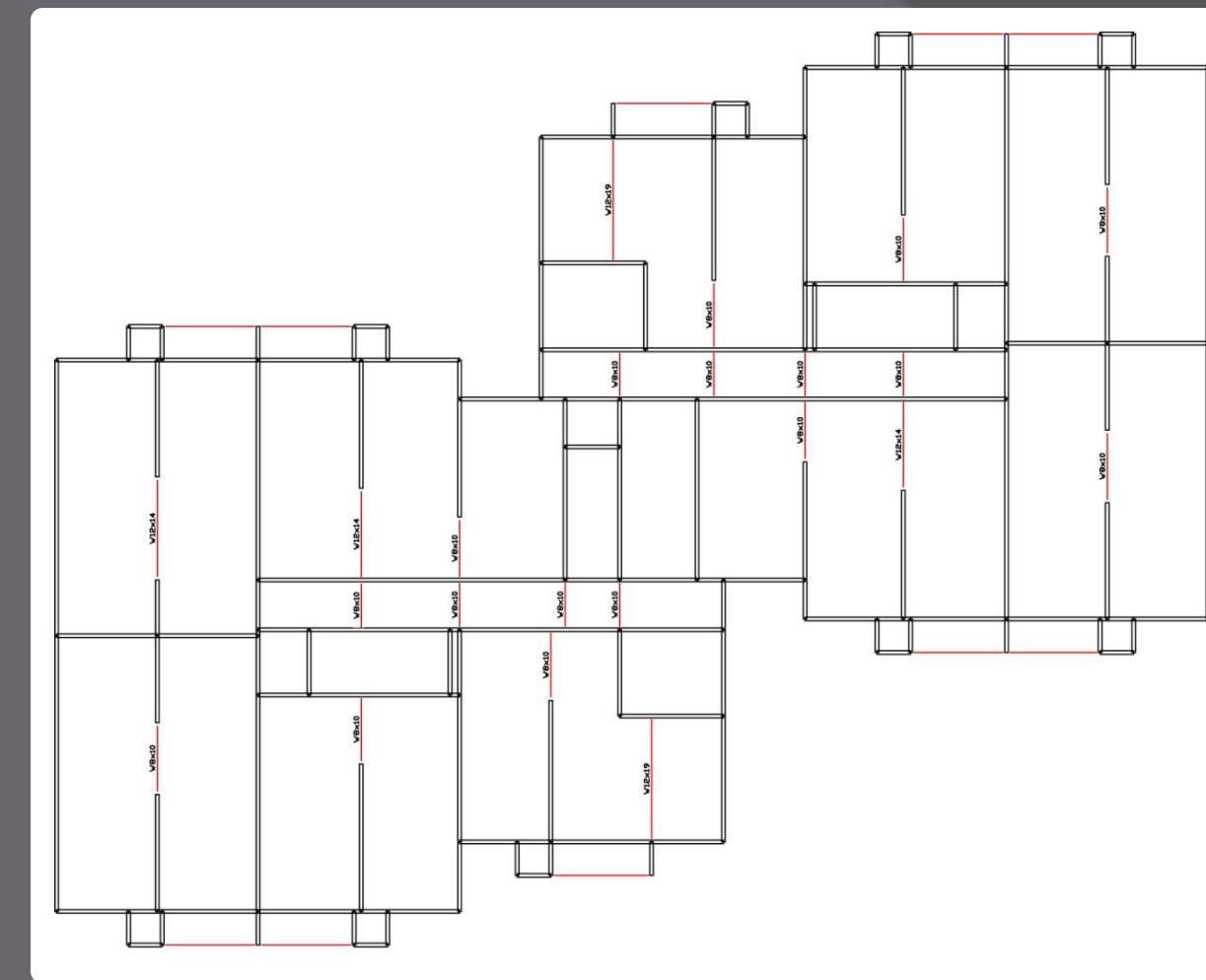


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Ground Level Layout



Typical Level Layout

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Floor System:

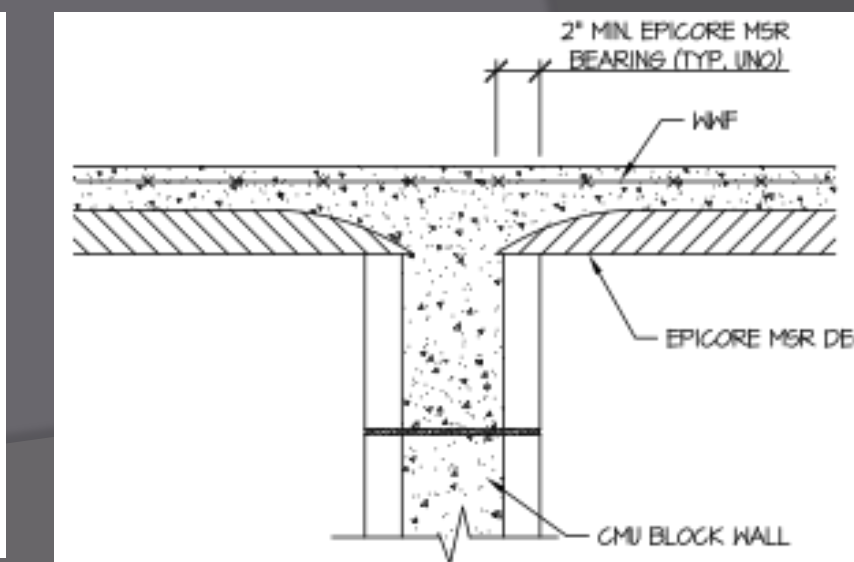
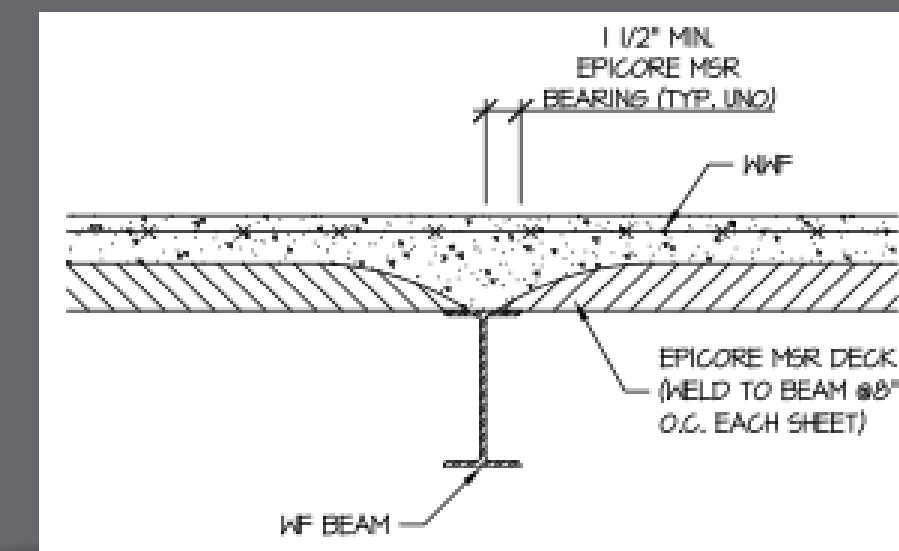
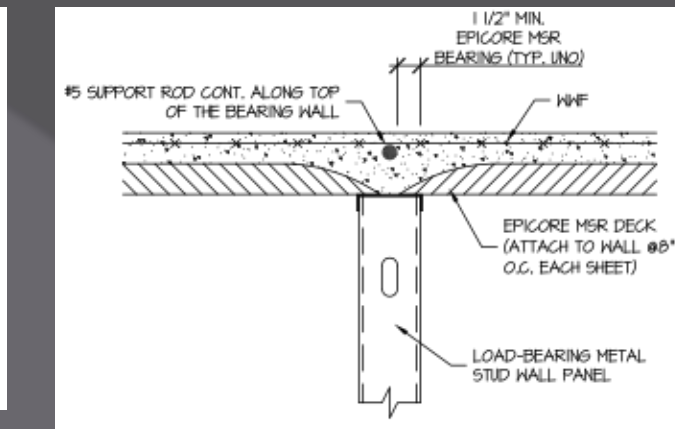
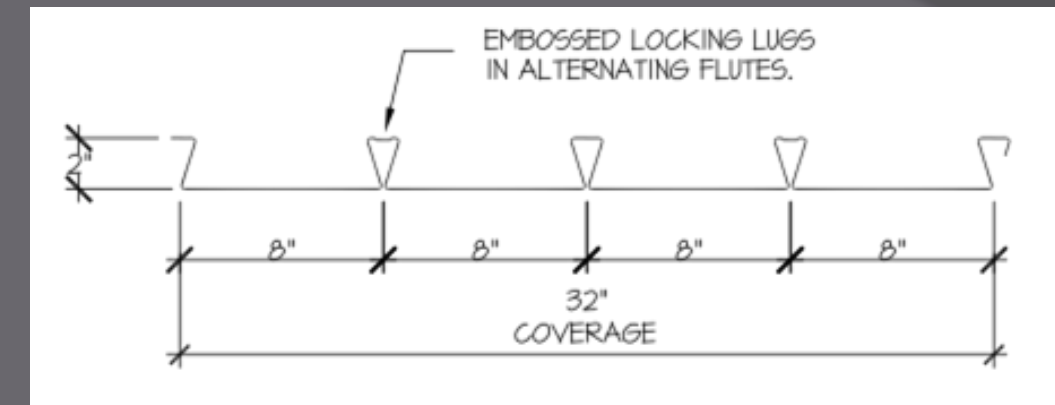
Slab on deck system

4 inch total system depth

Epicore MSR 20 gage composite deck

Dovetail shaped composite decking flutes provides sufficient reinforcement across spans without need for reinforcing bar

4000 psi concrete



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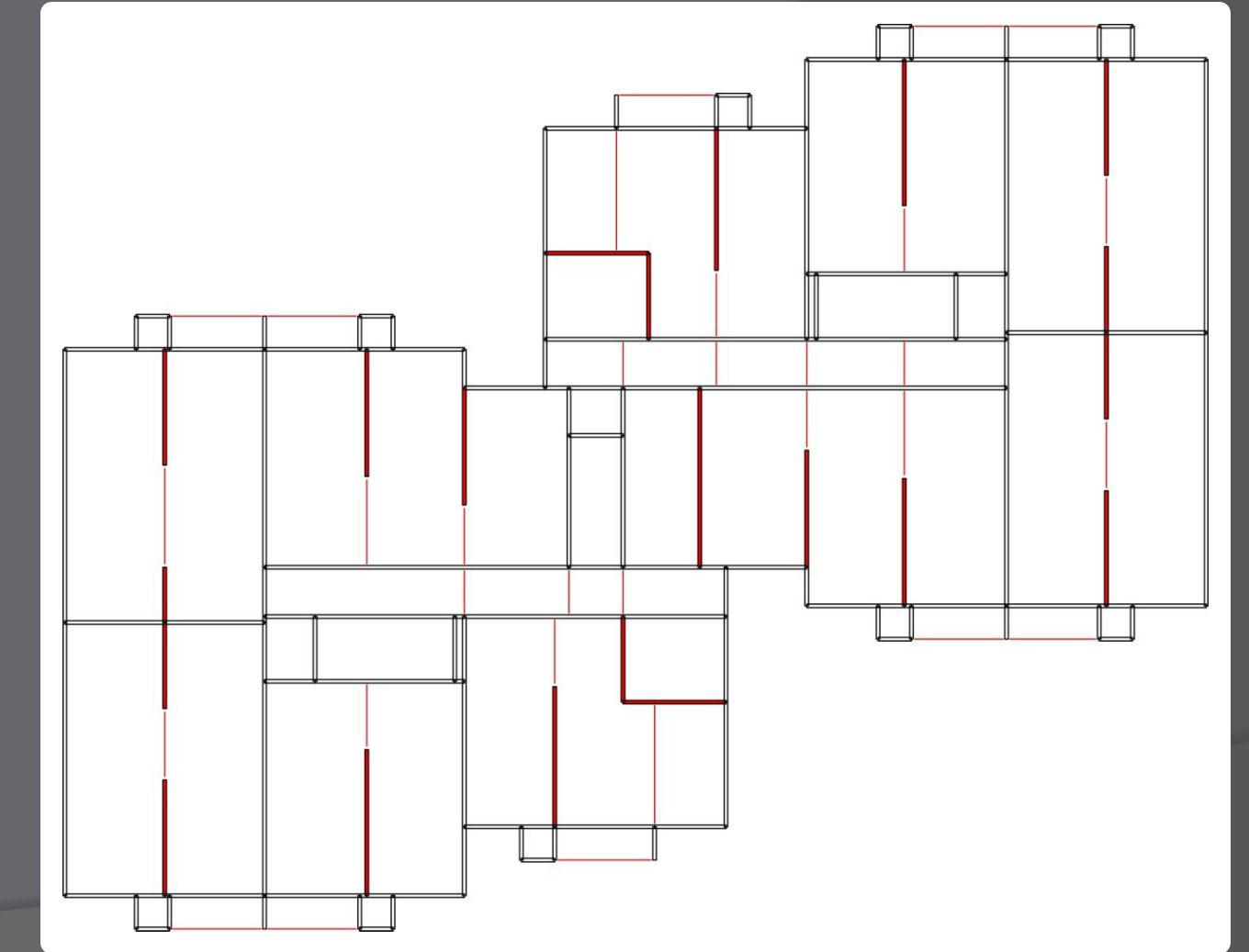
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Floor System:

Interior partition walls reassigned as load bearing



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Lateral System:

“Infinity Shear Panels” (ISP)

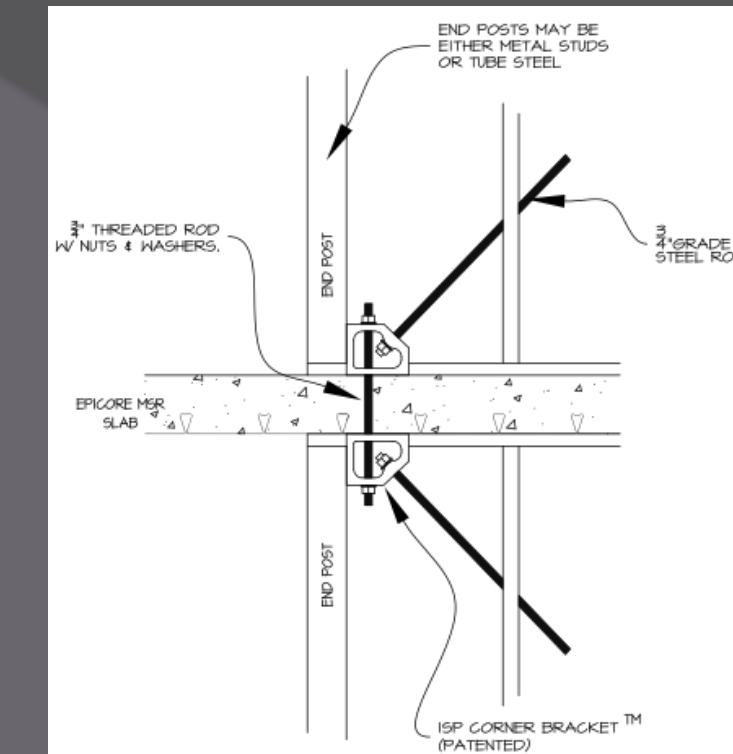
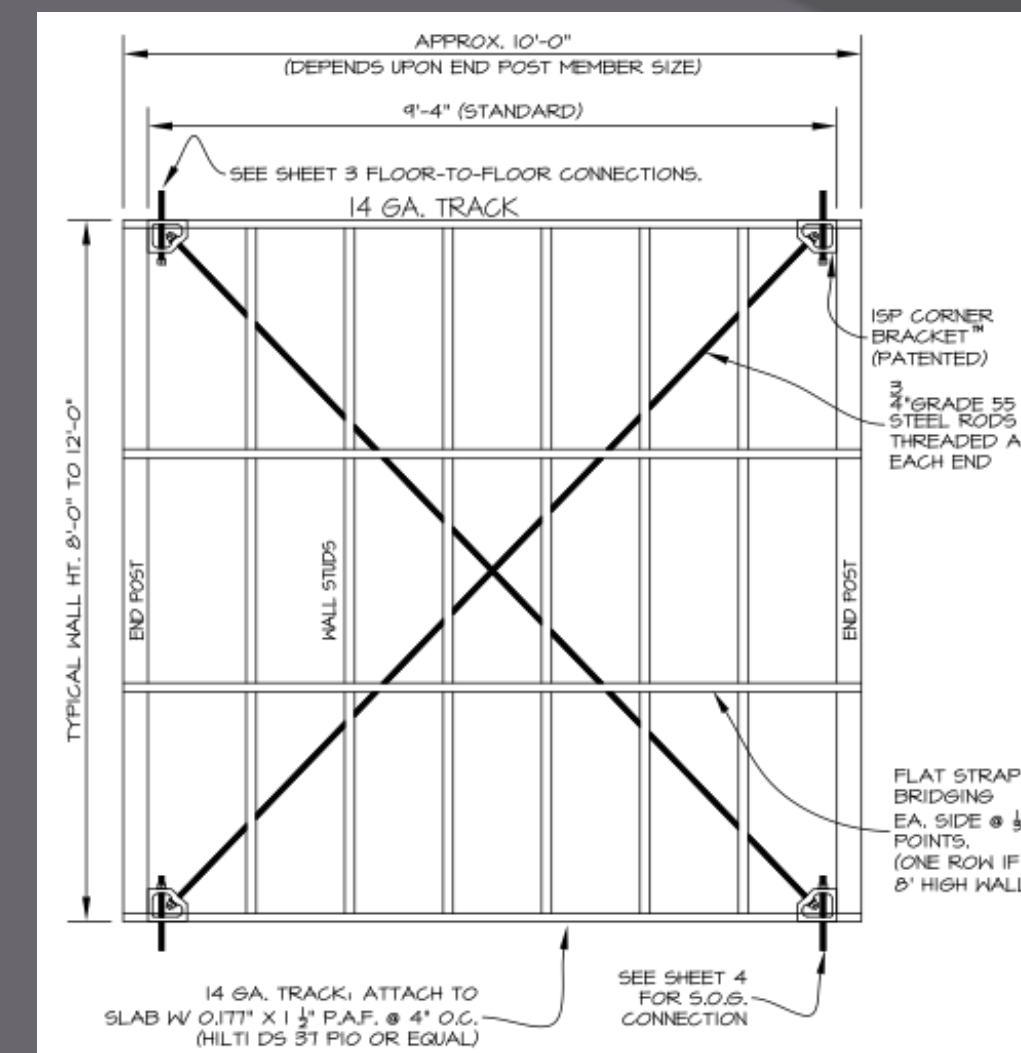
3/4 inch round threaded grade 55 steel rods

10 ft wide panels

Thru-bolted assembly from level to level for continuously braced lateral system

Rods inside wall, no surface bulging

Metal stud size and spacing match gravity system



ISP HEIGHT	ALLOWABLE
8'- 0"	9.40K
8'- 6"	9.13K
9'- 0"	8.87K
9'- 6"	8.61K
10'- 0"	8.36K
10'- 6"	8.13K
11'- 0"	7.90K
11'- 6"	7.68K
12'- 0"	7.47K

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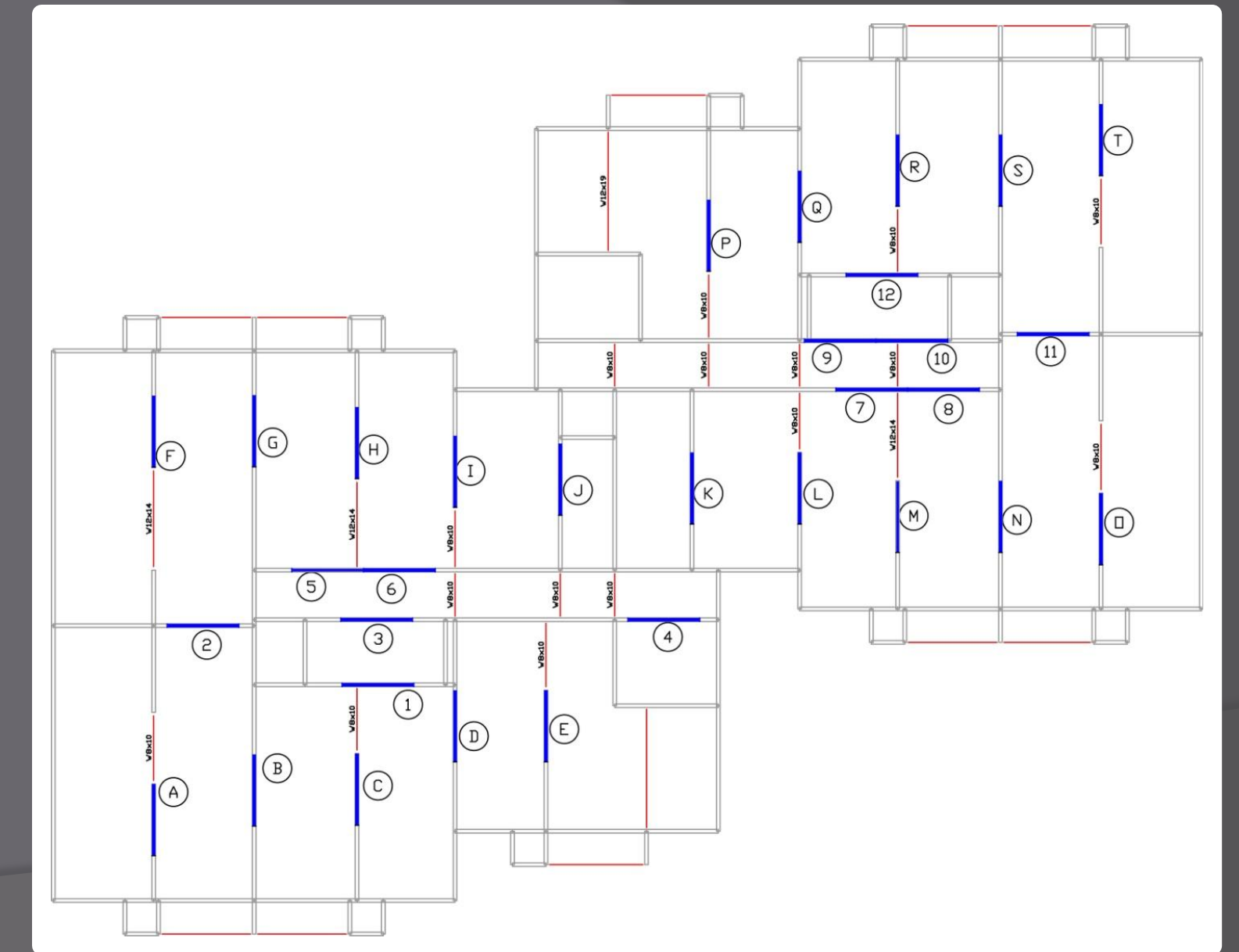
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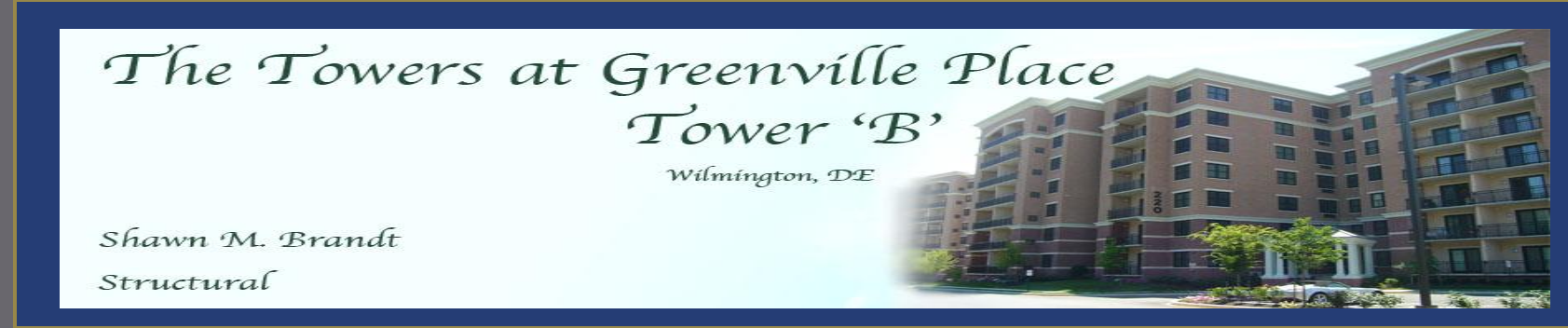
Lateral System:

Shear wall layout



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Weight Comparison:

Level	Dead Loads
Roof	821.30
7	1788.24
6	1738.89
5	1738.89
4	1738.89
3	1738.89
2	1738.89
1	1788.24
Ground	1685.56
Total Dead Load	14777.80

Existing Weight

Level	Dead Loads
Roof	1025.82
7	873.75
6	872.41
5	877.05
4	886.22
3	886.22
2	918.69
1	922.36
Ground	1503.82
Total Dead Load	8766.34

Redesigned Weight
(including wet green roof deal load)

Self weight Comparisons	
Existing	14,777.80 Kips
Redesigned	8,766.34 Kips
Difference	-6,011.46 Kips
Percent Reduction	41% Reduction

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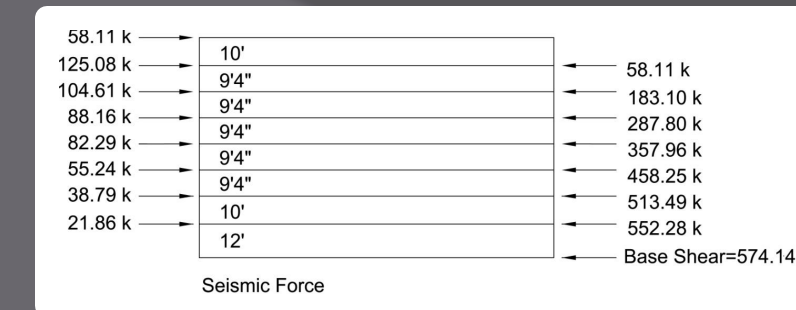
Seismic Comparison:

Level	Fx (Kips)	Vx (Kips)	Overturning Moment (ft-k)
Roof	58.11	-	-
7	125.08	58.11	3990.20
6	104.61	183.18	10868.27
5	88.16	287.80	14389.83
4	82.29	375.96	17545.99
3	55.24	458.25	14356.95
2	38.79	513.49	11296.82
1	21.86	552.28	6627.40
Ground	0.00	574.14	-
Total Overturning Moment=			79075.45

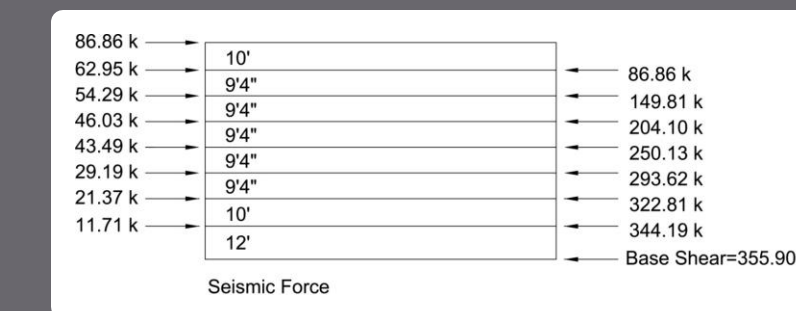
Existing Seismic

Level	Fx (Kips)	Vx (Kips)	Overturning Moment (ft-k)
Roof	86.86	-	-
7	62.95	86.86	5964.94
6	54.29	149.81	8888.20
5	46.03	204.10	10204.86
4	43.49	250.13	11673.57
3	29.19	293.62	9199.08
2	21.37	322.81	7101.89
1	11.71	344.19	4130.25
Ground	0.00	355.90	-
Total Overturning Moment=			57162.78

Redesigned Seismic



Existing Seismic



Redesigned Seismic

Seismic Comparisons	
Existing	574.14 Kips
Redesigned	355.90 Kips
Difference	-218.24 Kips
Percent Reduction	38% Reduction

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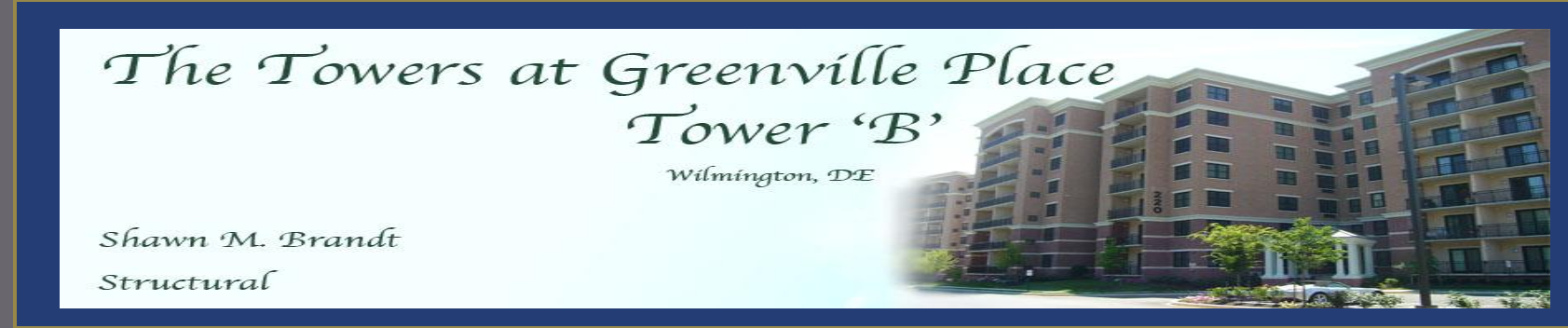
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Cost Comparison:

Quantity	Description	Ext. Mat. O&P	Ext. Labor O&P	Ext. Equip. O&P	Ext. Total O&P
1130 SFCA	C.I.P. concrete forms	9266	5650	0	14916
3390 C.Y.	Structural concrete, 3000 psi	376290	0	0	376290
480 C.Y.	Structural concrete, 4000 PSI	55680	0	0	55680
179014 Ea.	Concrete block	270311.14	0	0	270311.14
90895 S.F.	Precast slab	731704.75	154521.5	59081.75	945308
5.2 Ton	Reinforcing steel #4 to #7, footings	8450	5720	0	14170
7.7 Lb.	Reinforcing steel, #3 to #7, floors, walls	6.85	6.01	0	12.86
129.85 C.S.F.	Welded wire fabric 6 x 6 - W1.4 x W1.4	2577.52	4349.98	0	6927.5
362 L.F.	Concrete block, lintel	1339.4	2298.7	184.62	3822.72
Total		\$1455625.66	\$172546.19	\$59266.37	\$1,687,438.22

Existing Structural Costs

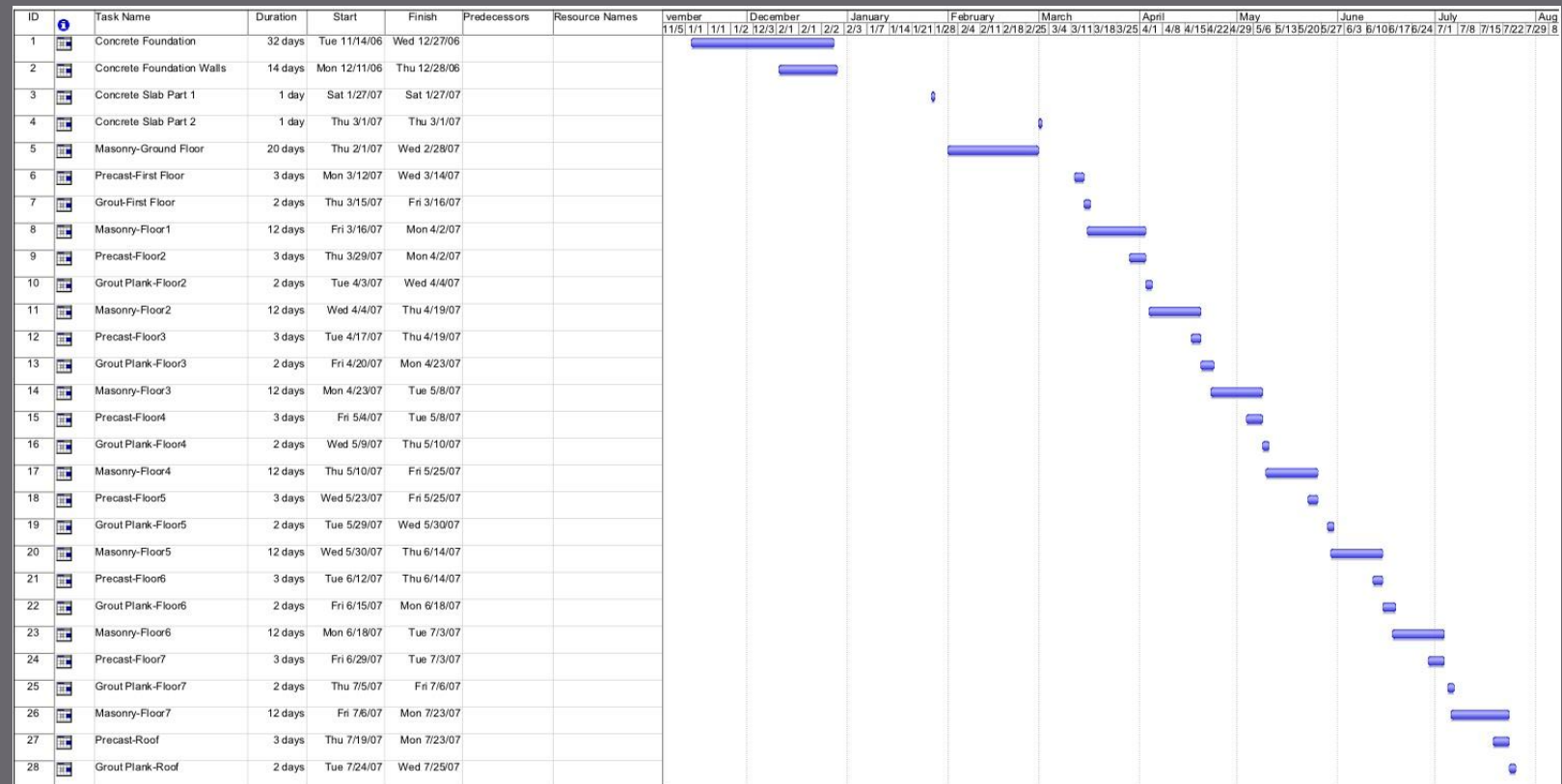
Quantity	Description	Ext. Mat. O&P	Ext. Labor O&P	Ext. Equip. O&P	Ext. Total O&P
4200 C.Y.	Structural concrete, 3000 psi	466200	0	0	466200
1130 SFCA	concrete forms	9266	5650	0	14916
10770 L.F.	Metal studs	200860.5	146472	0	347332.5
1039 C.S.F.	Welded wire fabric 6 x 6 - W1.4 x W1.4	20624.15	34806.5	0	55430.65
10.1 Ton	Reinforcing steel, footings, #4 to #7	16412.5	11110	0	27522.5
103880 S.F.	Metal decking	470576.4	78948.8	4155.2	553680.4
1642 L.F.	W8x10	29802.3	11411.9	5237.98	46452.18
58 L.F.	W18x40	4205	368.3	124.7	4698
530 L.F.	W12x16	15370	2512.2	1155.4	19037.6
16.5 L.F.	W16x26	775.5	68.81	31.68	875.99
20.08 L.F.	W16x31	1134.52	92.97	42.77	1270.26
25 L.F.	W18x35	1587.5	158.75	53.75	1800
3847 C.Y.	Structural concrete, 4000 PSI	446252	0	0	446252
82 L.F.	W10x45	6683	331.28	152.52	7166.8
36 L.F.	W12x87	5688	152.64	70.2	5910.84
44.3 Ton	Reinforcing steel, #7 to #11	2347.9	0	0	2347.9
Total		\$1697785.27	\$292084.14	\$11024.20	\$2,000,893.61

Redesigned Structural Costs

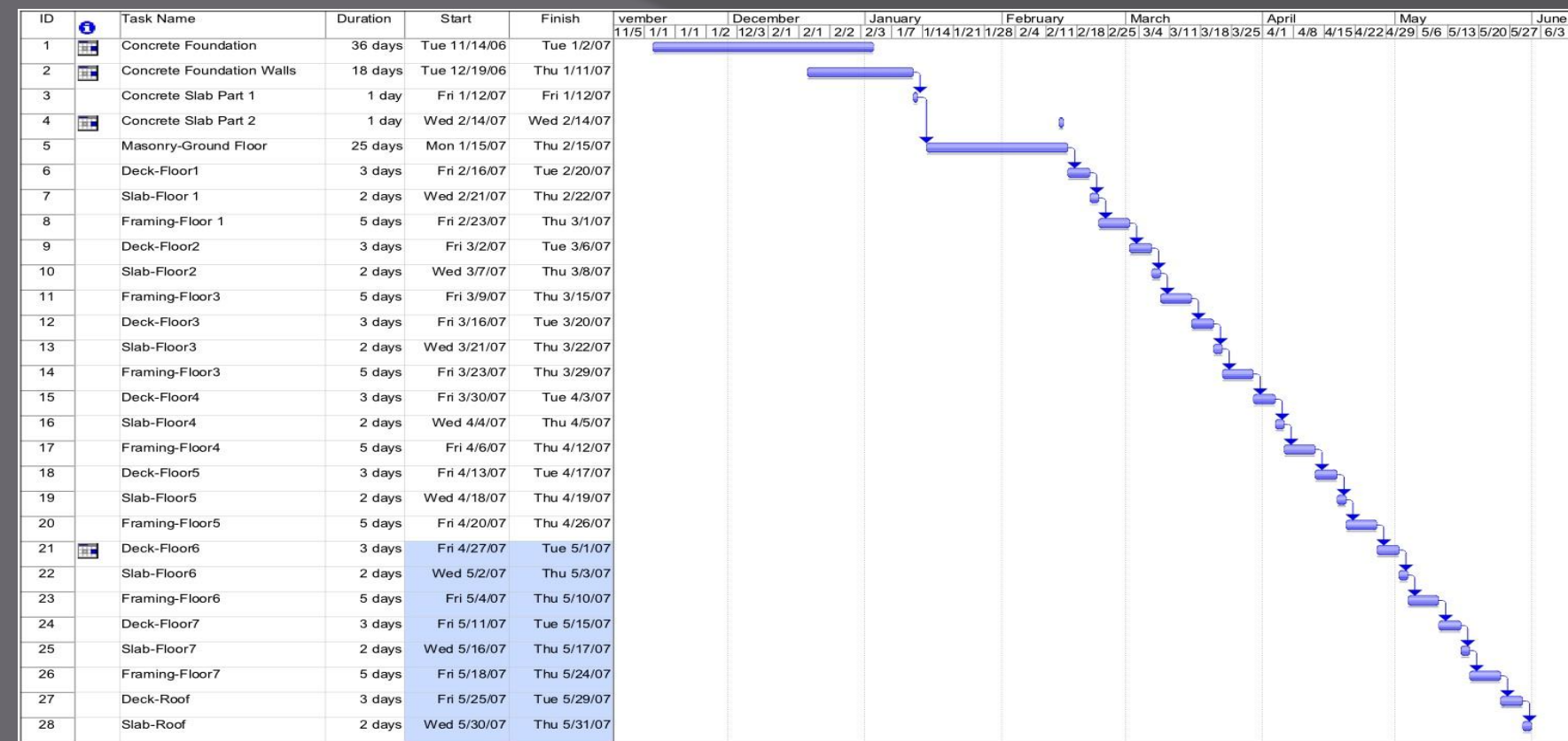
Structural Cost Comparisons	
Existing	\$1,687,438.22
Redesigned	\$2,000,893.61
Difference	+\$313,455.39
Percent Difference	19% Increase

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Existing Structural Construction Time



Redesigned Structural Construction Time

Structural Construction Time Comparison	
Existing	192 Days
Redesigned	156 Days
Difference	-36 Days
Percent Difference	19% Reduction

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Conclusions:

Depth Topic

Advantages:

- Successfully reduced weight and
- Seismic forces

Disadvantages:

- Partition walls must be reassigned as load bearing
- Columns placed in open areas

Conclusions:

Construction Management Breadth

Advantages:

- Faster construction time

Disadvantages:

- Greater cost of construction

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Questions & Comments